

Referências Bibliográficas

ALONSO, E. E.; GENS, A.; HIGHT, D. W. Special problem soils. General report. **Proc. 9th European Conf. Soil Mech. Fdn Engng.** Dublin, No. 3, 1987. pp 1087-1146.

ALONSO, E. E.; GENS, A.; JOSA, A. A constitutive model for partially saturated soils. **Geotechnique**, V 40, No. 3, 1990, pp 405-430.

AL-TABBAA, A. **Permeability and stress-strain response of speswhite kaolin.** PhD thesis, Cambridge University, 1987.

AL-TABBAA, A.; WOOD, D. M. An experimentally based 'bubble' model for clay. **Int. Conf. Num. Models Geomech. NUMOG III.** Pietruszczak & Pande (eds), Balkema, 1989.

ARMALEH, S. H.; DESAI, C. S. Modelling and Testing of a Cohesionless Soil Using the Disturbed State Concept. **Int. Journal of Mechanical Behavior of Materials**. V. 5, No 3, 1994.

BALMACEDA, A.; JOSA, A.; GENS, A.; ALONSO, E. E. An elastoplastic model for partially saturated soils exhibiting a maximum collapse. **Proc. 3rd Int. Conf. On Comp. Plasticity**. V. 1, Barcelona, 1992.

BARROSO DE MELO, L. T. **Utilização de um Modelo Elasto-Plástico para a Análise de Deformações em Solos.** Dissertação de Mestrado, Dpto. Eng. Civil, PUC-RIO, 1995.

BRENNER, R. P.; GARGA, V. K.; BLIGHT, G.E. Shear strength behavior and the measurement of shear strength in residual soils. **Mechanics of residuals soils**. Blight (ed), ch. 9, Rotterdam: Balkema, 1997.

CAMPELO, N. S. **Comportamento de estacas submetidas a esforços verticais de tração, em solo colapsível.** Dissertação de Mestrado, EESC/USP, 1994.

CARNEIRO, B. J. I. **Comportamento de estacas apiloadas em solo colapsível.** Dissertação de Mestrado, EESC/USP, 1994.

CHEN, W. F.; BALADI, G. Y. **Soil Plasticity: Theory and Implementation.** New York: ELSEVIER, 1985.

CHEN, W. F.; HAN, D. J. **Plasticity for Structural Engineers.** Springer-Verlag, 1988.

COLEMAN, J. D. Stress strain relations for partly saturated soil. **Geotechnique**. V 12, No. 4, 1962. pp 348-350.

CUI, Y. J.; DELAGE, P. Yielding and plastic behavior of an unsaturated compacted silt. **Geotechnique**. V. 46, No 2, 1996. pp 291-311.

DAFALIAS, Y. F. **On cyclic and anisotropic plasticity. (i) A general model including material behavior under stress reversals: (ii) Anisotropic hardening for initially orthotropic materials.** PhD thesis, University of California, Berkeley, 1975.

DAFALIAS, Y. F. Bounding Surface Plasticity. I: Mathematical foundation and Hipoplasticity. **Journal of Engineering Mechanics**. Vol. 112, No 9, 1986. pp 966-987.

DAFALIAS, Y. F. An anisotropic critical state clay plasticity model. In proc. 2nd Int. Conf. On Constitutive Laws for Engineering materials. V 1, Tucson, Arizona: Elsevier, 1987. pp 513-521.

DAFALIAS, Y. F.; PAPOV, E. P. Plastic internal variables formalism of cyclic plasticity. **Jnl. Appl. Mech.** V 98, No. 4, 1976. pp 645-650.

DESAI, C. S. A Consistent Finite Element Technique for Work-Softening Behavior. **Proc. Int. Conf. On Comp. Meth. In Nonlinear Mech.** J.T. Oden (ed), Univ. of Texas, Austin, 1974.

DESAI, C. S. A general basis for yield, failure and potencial functions in plasticity. **Int. Journal of Numerical and analytical Methods in Geomechanics**. V. 4, 1980. pp 361-375.

DESAI, C. S. **Mechanics of Materials and Interfaces: The disturbed State Concept.** Boca Raton, Florida: CRC Press, 2000.

DESAI, C. S.; FARUQUE, O. Constitutive models for geological materials. **Journal of Engineering Mechanics**. V. 110, No. 9, 1984. pp 1391-1408.

DESAI, C. S.; SIRIWARDANE, H. J. **Constitutive laws for Engineering Materials, with emphasis on Geologic Materials.** New Jersey: Prentice-Hall, 1984.

DESAI, C. S.; TOTH, J. Disturbed state constitutive modeling based on stress-strain and nondestructive behavior. **Int. Journal of solid Structures**. V. 33, No 11, 1996. pp 1619-1650.

DESAI, C. S.; SOMASUNDARAM, S.; FRANTZISKONIS, G. A hierarquical approach for constitutive modeling of geologic materials. **Int. Journal of Numerical and analytical Methods in Geomechanics**. V. 10, 1986. pp 225-257.

DESAI, C. S.; WATHUGALA, G. N. **Hierarquical and Unified Models for Solids and Discontinuities.** Notes for short course. Tucson, Arizona, 1987.

DiMAGGIO, F. L.; SANDLER, I. S. Material model for Granular Soils. **Journal of Eng. Mech. Div. ASCE**, V 97, No. EM3, 1971. pp 935-950.

DRUCKER, D. C. A more fundamental Approach to Plastic Stress-Strain Relations. **1st U.S. Congress of Applied Mechanics**. New York: ASME, 1952. pp 487-491.

DRUCKER, D. C.; PRAGER, W. Soil Mechanics and Plastic Analysis or Limit Design. **Quart. Appl. Math.**, V10, No. 2, 1952. pp 157-165.

DRUCKER, D. C.; GIBSON, R. E.; HENKEL, D. J. **Soil Mechanics and work hardening theories of plasticity**. Trans. ASCE, V 122, 1957. pp 338-346.

DUNCAN, J. M. Hyperbolic Stress-Strain Relationships. **Proceedings of the Workshop in Limit Equilibrium, Plasticity and Generalized Stress-Strain in Geotechnical Engineering**. R.K. Yong & H.Y. Ko (eds), ASCE, 1980. pp 443-460.

DUNCAN, J. M.; CHANG, Y. C. **Nonlinear analysis of stress and strain in soils**. ASCE, SM5, Vol 96, 1970. pp 1629-1653.

DUNCAN, J. M.; BYRNE, P.; WONG, K. S.; MABRY, P. Strength, Stress-Strain and Bulk Modulus Parameters for Finite Element Analyses of Stress and Movement in Soil Masses. **Geotechnical Engineering Research Report**. UCB/GT 80-01, Department of Civil Engineering, University of California, Berkeley, 1980.

DUNCAN, J. M.; SEED, R. B.; WONG, K. S.; OZAWA, Y. **FEADAM84: A Computer Program for Finite Element Analysis of Dams**. Department of Civil Engineering, Virginia Polytechnic Institute and State University, Blacksburg, 1984.

ERINGEN, A. C. **Nonlinear Theory of Continuous Media**. Englewood Cliffs, N.J: McGraw-Hill, Inc., 1962.

ESCARIO, V.; SAEZ, J. Shear strength of soils under high suction values. **IX ECSMFE**. Dublin, 1987.

FREDLUND, D. G.; MORGENSTERN, N. R. Stress state variables for unsaturated soils. **J. Geotech. Engng. Div. ASCE**, V 103, GT5, 1977. pp 447-456.

FREDLUND, D. G.; MORGENSTERN, N. R.; WIDGER, R. A. The shear strength of unsaturated soils. **Canadian Geotechnical Journal**. No 15, 1978. pp 313-321.

GEISER, F.; LALOUI, L.; VULLIET, L. Disturbed state concept for partially saturated soils. **Numerical models in Geomechanics**. Pietruszczak & Pande (eds), Rotterdam: Balkema, 1997. pp 129-133.

GENS, A.; Nova, R. Conceptual bases for a constitutive model for bonded soil and weak rocks. **Geotechnical Engineering of Hard Soils – Soft Rocks**. Anagnostopoulos et al (eds), Rotterdam: Balkema, 1993. pp 485-494.

GENS, A.; POTTS, D. M. A theoretical model for describing the behavior of soils not obeying Rendulic's principle. **Proc. 1st Int. Symp. Numer. Mod. Geomech.** Zurich, 1982. pp 24-32.

GERRARD, C. M. & HARRISON, W. J. Circular loads applied to a cross-anisotropic half-space. C.S.I.R.O. Aust. Div. App. Geomech. Tech. Pap. N°8, 1970.

HERRMANN, L. R. **User's Manual for REA** (General Two Dimensional Soils and Reinforced Earth Analysis Program. Department of Civil Engineering Report, University of California, Davis, 1978.

HOEK, E.; BROWN, E. T. Empirical Strength Criterion for Rock Masses. **Journal of the Geotechnical Engineering division.** V. 106, No. GT9, 1980. pp 1013-1035.

HOULSBY, G. T. The use of variable shear modulus in elastic-plastic models for clays. **Comput. Geotechn.**, V 1, 1985. pp 3-13.

HVORSLEV, M. J. Physical Properties of Remolded Cohesive Soils. **Transl. 69-5.** Vicksburg, Miss: U.S. Army Corps of Engineers Waterways Exp. Stn., June 1969.

JANBU, N. Soil Compressibility as Determined by Oedometer and Triaxial Tests. **Proceedings European Conference on Soil Mechanics and Foundation Engineering.** Wiesbaden, V1, 1963. pp 19-25.

KACHANOV, L. M. **Introduction of Continuum Damage Mechanics.** Dordrecht, The Netherlands: Martinus Nijhoff, 1986.

KATTI, D. R.; DESAI, C. S. Modelling and Testing of a Cohesive Soils Using the Disturbed State Concept. **Int. Journal of Eng. Mechanics.** ASCE, V. 121, No 5, 1995. pp 648-658.

KAVVADAS, M.; AMOROSI, A. A constitutive model for structured soils. **Geotechnique.** V 50, No. 3, 2000. pp 263-273.

KLISINSKI, M. Degradation and Plastic Deformation of Concrete. **IFTR Report 38.** PhD Thesis, Polish Academy of Sciences, 1985.

KOLYMBAS, D. A rate-dependent constitutive equation for soils. **Mech. Research Communications.** V. 4, 1977. pp 367-372.

KOLYMBAS, D. A generalized hipoelastic constitutive law. **Proc. XI Conf. Soil Mechanics and Foundation Engineering.** V. 5, 1985. pp 2626.

KOLYMBAS, D. **Introduction to Hipoplasticity.** Rotterdam: A.A. Balkema, 2000.

KOLYMBAS, D.; HERLE, I.; VON WOLFFERSDORFF, P. A. Hypoplastic Constitutive Equation with Internal Variables. **Int. Journal for Num. and Anal. Methods in Geomechanics**. V. 19, No 6, 1995. pp 415-436.

KONDNER, R. L. **Hiperbolic stress-strain response**: cohesive soils. ASCE, SM1, Vol 82, 1963. pp 115-143.

KRIEG, R. D. A practical two-surface plasticity theory. **Jnl. Appl. Mech.** V 42, 1975. pp 641-646.

KULHAWY, F. H.; DUNCAN, J. M.; SEED, H. B. Finite element analysis of stresses and movements in Embankments during construction. **Rep 569-8**. Vicksburg, Miss.: U.S. Army Corps of Engineers Waterways Exp. Stn., 1969.

LADE, P. V. Elasto-plastic stress-strain theory for cohesionless soil with curved yield surfaces. **Int. Jnl. Solids Struct.** V 13, 1977. pp 1019-1035.

LADE, P. V.; DUNCAN, J. H. Elastoplastic Stress-Strain Theory for Cohesionless Soil. **Journal of Geotechnical Engineering**. ASCE, V. 101, No 10, 1975. pp 1037-1053.

LADE, P. V.; KIM, M. K. Single hardening constitutive model for frictional materials. I: Plastic potencial function. **Computers and Geotechnics**. V 5, 1988. pp 307-324.

LADE, P. V.; KIM, M. K. Single hardening constitutive model for frictional materials. II: Yield Criterion and plastic work contours. **Computers and Geotechnics**. V 6, 1988. pp 13-29.

LADE, P. V.; KIM, M. K. Single hardening constitutive model for frictional materials. III: Comparisons with experimental data. **Computers and Geotechnics**. V 6, 1988. pp 31-47.

LIU, M. D.; CARTER, J. P. Modelling the destructuring of soils during virgin compression. **Geotechnique**. V 50, No. 4, 2000. pp 479-483.

LIU, M. D.; CARTER, J. P. A structured Cam Clay model. **Canadian Geotechnical Journal**. V 39, No. 6, 2002. pp 1313-1332.

LLORET, A.; ALONSO, E. E. State surface for partially saturated soils. **Proc. 11th Int. Conf. Soil Mech. Fdn. Engng.** San Francisco, V 2, 1985. pp 557-562.

MACHADO, S. L. **Aplicação de conceitos de elastoplasticidade a solos não saturados**. Tese de Doutorado, Escola de Engenharia de São Carlos, Univ. de São Paulo, 1998.

- MATSUOKA, H. Stress-Strain relationship of sands based on the Mobilized Plane. **Soils and Foundations**. V 14, No. 2, June 1974. pp 47-61.
- MATSUOKA, H.; NAKAI, T. Stress-Deformation and Strength Characteristics of Soil Under Three Different Principal Stresses. **Proc. Jap. Soc. Civil Engrs.** No. 232, 1974. pp 59-70.
- MATSUOKA, H.; NAKAI, T. Stress-strain relationship of soil based on the Spatial Mobilized Plane. **IX ICSMFE**. in Proceedings Specialty Session 9, Constitutive Equations of Soils, 1977. pp 153-162.
- MATSUOKA, H. Constitutive Equation and FE analysis for Anisotropic Soil. **Int. Cong. Num. Meth. Geomech.** Edmonton, 1982. pp 223-233.
- MATYAS, E. L.; RADHAKRISHNA, H. S. Volume change characteristics of partially saturated soils. **Geotechnique**. V 18, No. 4, 1968. pp 432-448.
- MEDINA, J. D. Propriedades mecânicas dos solos residuais. **Seminário de Mecânica dos solos do Programa de Eng. Civil**. Rio de Janeiro: COPPE, UFRJ, N° 2, 1970.
- MENDELSON, A. **Plasticity: Theory and Application**. New York: The Macmillan Company, 1968.
- MENÉTREY, Ph. **Numerical Analysis of Punching Failure in Reinforced Concrete Structures**. Thesis, Ecole Polytechnique Fédérale de Lausanne, 1994.
- MENÉTREY, Ph.; Willam, K. J. Triaxial Failure Criterion for Concrete and Its Generalization. **ACI Structural Journal** V. 92, No 3, 1995. pp 311-318.
- MOURATIDIS, A.; MAGNAN, J. P. Un modele elastoplastique anisotrope avec ecrouissage pour les argilles molles naturelles: Melanie. **Rev. Fr. Geotechn.** V 25, 1983. pp 55-62.
- MROZ, Z. On the Description of Anisotropic Work Hardening. **J. Mech. Phys. Solids**. V 15, 1967. pp 163-175.
- MURAYAMA, S. A theoretical consideration on a behavior of sand. **Proc. Of IUTAM Symposium on Rheology and Soil Mechanics**. Grenoble, 1964. pp146-159.
- NAKAI, T.; MATSUOKA, H. Shear behaviors of sand and clay under three-dimensional stress condition. **Soils and Foundations**. V 23, No. 2, 1983. pp 26-42.
- NAYAK, G. C.; ZIENKIEWICZ, O. C. A convenient form of Invariants and Its Application in Plasticity. **Proc. ASCE**. V 98, No. ST4, 1972. pp. 949-954

NAYLOR, D. J.; PANDE, G. N. **Finite elements in geotechnical engineering**. Swansea, UK: Pineridge Press, 1981.

NELSON, J.; BARON, M.L. Application of variable moduli to soil behavior. **Int. J. Solids and Structures**. V7, 1971. pp 399-417.

NOGUEIRA, C. L. **Análise Não-Linear de Escavações e Aterros**. Tese de Doutorado, Dpto. Eng. Civil, PUC-RIO, 2001.

OLIVEIRA, C. P. **Estudo do comportamento tensão-deformação-resistência de um solo residual de biotita gnaiss saturado**. Dissertação de Mestrado, Dpto. Eng. Civil, PUC-RIO, 2000.

OLIVEIRA, A. M. S.; BRITO, S. N. A. **Geologia de Engenharia**. São Paulo: ABGE, 1998.

OHMAKI, S. Stress-strain behavior of anisotropically, normally consolidated cohesive soil. **Proc. 1st Int. Symp. Num. Mod. Geomech.** Zurich, 1982. pp 250-269.

PLAXIS, **Finite Element Code for Soil and Rock Analyses**. Version 7, A.A. Rotterdam: Balkema, 1998.

POTTS, D. M.; ZDRAVKOVIC, L. **Finite element analysis in geotechnical engineering: Theory**. Heron Quay, London: Thomas Telford Ltd., V1, 1999.

PRAGER, W. Recent Developments in Mathematical Theory of Plasticity. **J. Appl. Phys.** V 20, No.3, Mar. 1949. pp 235-241.

PRAGER, W. A new Method in Analyzing Stresses and Strains in Work-Hardening Plastic Solids. **J. Appl. Mech.** V 78, 1956. p 493.

PREVOST, J. H. Mathematical modeling of Monotonic and Cyclic Undrained Clay Behavior. **Int. J. Numer. Anal. Methods Geomech.** V 1, No. 2, 1977. pp 195-216.

PREVOST, J. H. Anisotropic Undrained Stress-strain Behavior of Clays. **J. Geotech. Eng. Div. ASCE**, V 104, No. GT8, Aug. 1978. pp 1075-1090.

PREVOST, J. H. Mathematic modeling of Soil Stress-Strain Strength Behavior. **Proc. 3rd Int. Conf. Numer. Methods Geomech.** Aachen, W. Germany, W. Wittke (Ed.), V 1, Rotterdam: Balkema Press, 1979. pp 347-361.

RENDULIC, L. Pore-Index and Pore Water Pressure, **Bauingenieur**. V17, No. 559, 1936.

ROSCOE, K. H.; SCHOFIELD, A.; WORTH, C. P. On the Yielding of Soils. **Geotechnique**. V 8, 1958. pp 22-53.

ROSCOE, K. H.; SCHOFIELD, A.; e THURAIRAJAH, A. Yielding of Clays in State Wetter than Critical. **Geotechnique**. V13, 1963. pp 12-28.

ROSCOE, K. H.; BURLAND, J. B. On the generalized Stress-Strain Behavior of 'wet' Clay. in **Engineering Plasticity**. J. Heyman and F. A. Leckie (Eds.), Cambridge: Cambridge University Press, 1968. pp 535-609.

ROWE, P. W. The stress-dilatancy relation for static equilibrium of an assembly of particles in contact. **Proc. Roy. Soc. A**. No.269, 1962. pp 500-527.

ROUAINIA, M.; MUIR WOOD, D. A kinematic hardening model for natural clays with loss of structure. **Geotechnique**. V 50, No. 2, 2000. pp 153-164.

SABOYA Jr., F. A.; Byrne, P. M. Parameters for Stress and Deformation Analysis of Rockfill Dams. **Canadian Geotechnical Journal**. V 30, No. 4, 1993. pp 690-701.

SANDLER, I. S.; DiMAGGIO, F. L.; BALADI, G. Y. Generalized Cap Model for Geologic Materials. **J. Geotech. Eng. Div. ASCE**, V 102, No. GT7, July 1976. pp 683-699.

SANDRONI, S. S.; MACCARINI, M. Ensaio triaxiais e de cisalhamento direto em uma ocorrência de solo residual gnáissico jovem rico em feldspato. **I Simpósio Brasileiro de Solos Tropicais em Engenharia**. COPPE/UFRJ, Rio de Janeiro, 1981, pp 324-339.

SCHANZ, T.; BONNIER, P. G. Verification of a soil model with predicted behavior of a sheet pile wall. **Computer Methods and Advances in Geomechanics**. Yuan (ed), Rotterdam: Balkema, 1997. pp 953-959.

SCHOFIELD, A. N.; WROTH, C. P. **Critical State Soil Mechanics**. London: McGraw-Hill Book Company, 1968.

SHIELD, R. T.; ZIEGLER, H. On Prager's Hardening Rule. **Z. Angew. Math. Phys.** V 9, 1958. pp 260-276.

STALLEBRASS, S. E.; TAYLOR, R. N. The development and evaluation of a constitutive model for the prediction of ground movements in overconsolidated clay. **Geotechnique**. V 47, No. 2, pp 235-254. 1997

TIMOSHENKO, S. P.; e GOODIER, J. N. **Theory of Elasticity**. New York: McGraw-Hill Book Company, 1970.

TRUESDELL, C. Hypoelasticity. **J. Ration. Mech. Anal.** Vol 4, 1955. pp 83-133.

VARGAS, M. **Introdução à mecânica dos solos**. São Paulo: McGraw-Hill do Brasil (USP), 1977.

VEIGA PINTO, A. A. Previsão do comportamento estrutural de barragens de enrocamento. **LNEC International Report**. Lisbon: Laboratório Nacional de Engenharia Civil, 1983.

WATHUGALA, G. W.; DESAI, C. S. Damage based constitutive model for soils. **Proc. 12th Canadian Congress of Applied Mechanics (CANCAM 89)**. Ottawa, 1989. pp 530-531.

WEIHE, S. **Implicit Integration Schemes for Multi-Surface Yield Criteria Subjected to Hardening/Softening Behavior**. MS Thesis, University of Colorado-Boulder, 1989.

WHITTLE, A. J. **A constitutive model for overconsolidated clays with application to the cyclic loading of friction piles**. PhD thesis, Massachusetts Institute of Technology, 1987.

WHITTLE, A. J.; KAVVADAS, M. J. Formulation of MIT-E3 constitutive model for overconsolidated clays. **Journal of Geotechnical Engineering**. ASCE, V 120, No. 1, 1994. pp 199-224.

WU, W. & BAUER, E. A simple Hypoplastic Constitutive /model for Sand. **Int. Journal for Num. and Anal. Methods in Geomechanics**. V. 18, No 12, 1994. pp 833-862.

WU, W.; BAUER, E.; KOLYMBAS, D. Hipoplastic constitutive model with critical state for granular materials. **Mechanics of materials**. V 23, 1996. pp 45-69.

ZIENKIEWICZ, O. C.; NAYLOR, D. J. Finite element studies of soils and porous media. **Lect. Finite elements in continuum mechanics**. Edt. Oden & de Arantes, UAH press, 1973. pp 459-493.

ZYTINSKI, M. On modelling the unloading-reloading behavior of soils. **Int. Journal Num. Anal. Meth. Geomech**. V 2, 1978. pp 87-93.

Apêndice

Tensores e espaços de tensões e deformações

Tensor de tensões. Entende-se por tensor um ente matemático que é independente do referencial. As constantes são tensores de ordem zero e os vetores são tensores de ordem 1.

A figura A1 apresenta as componentes positivas de tensão que atuam em um elemento diferencial de solo.

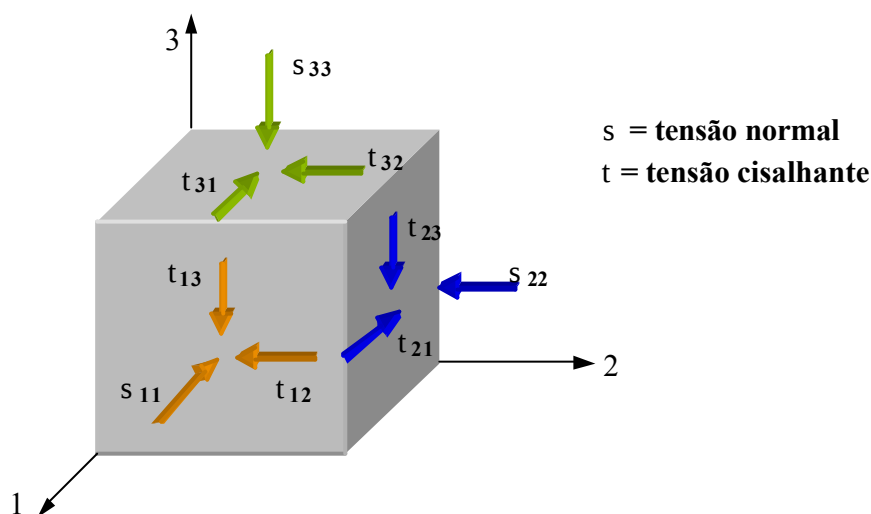


Figura A1: Convenção de tensões num elemento diferencial de solo.

O estado de tensões esquematizado na figura A1 pode ser expresso através do tensor de tensões,

$$T_s = \begin{bmatrix} s_{11} & t_{12} & t_{13} \\ t_{21} & s_{22} & t_{23} \\ t_{31} & t_{32} & s_{33} \end{bmatrix} \quad \text{ou} \quad T_s = \begin{bmatrix} s_{11} & s_{12} & s_{13} \\ s_{21} & s_{22} & s_{23} \\ s_{31} & s_{32} & s_{33} \end{bmatrix} \quad (A1)$$

Para um dado estado de tensões, o tensor T_s pode ser expresso como um somatório dos tensores *hidrostático* e *de desvio*, respectivamente, onde $p = \frac{1}{3}(s_{11} + s_{22} + s_{33})$

$$T_s = \begin{bmatrix} p & 0 & 0 \\ 0 & p & 0 \\ 0 & 0 & p \end{bmatrix} + \begin{bmatrix} s_{11} - p & t_{12} & t_{13} \\ t_{21} & s_{22} - p & t_{23} \\ t_{31} & t_{32} & s_{33} - p \end{bmatrix} \quad (A2)$$

Empregando-se a notação do delta de Kronecker, a equação A2 também pode ser expresso como abaixo, onde S_{ij} representa o tensor de desvio.

$$T_s = p d_{ij} + S_{ij} \quad (A3)$$

Invariantes do tensor de tensões - Existem duas maneiras para se obter os invariantes de tensão. A utilizada neste trabalho, constrói os invariantes a partir do próprio tensor, de acordo com as definições seguintes:

$$J_1 = tr(s) = s_{11} + s_{22} + s_{33} \quad (A4)$$

$$J_2 = \frac{1}{2} tr(s)^2 \quad (A5)$$

$$J_3 = \frac{1}{3} tr(s)^3 \quad (A6)$$

Quanto aos invariantes do tensor de tensões de desvio, estes são definidos como:

$$J_{1D} = tr(S) = 0 \quad (A7)$$

$$J_{2D} = \frac{1}{2} tr(S)^2 = \frac{1}{2} [(s_{11} - p)^2 + (s_{22} - p)^2 + (s_{33} - p)^2 + 2S_{12}^2 + 2S_{23}^2 + 2S_{13}^2] \quad (A8)$$

$$J_{3D} = \frac{1}{3} tr(S)^3 = J_3 - \frac{2}{3} J_1 J_2 + \frac{2}{27} J_1^3 \quad (A9)$$

Tensor de Deformações - O tensor de deformações pode ser obtido por analogia com o tensor das tensões, substituindo-se s por e , e $\frac{1}{2}g$ por t . Logo, temos

$$T_e = \begin{bmatrix} e_{11} & \frac{1}{2}g_{12} & \frac{1}{2}g_{13} \\ \frac{1}{2}g_{21} & e_{22} & \frac{1}{2}g_{23} \\ \frac{1}{2}g_{31} & \frac{1}{2}g_{32} & e_{33} \end{bmatrix} \quad \text{ou} \quad T_e = \begin{bmatrix} e_{11} & e_{12} & e_{13} \\ e_{21} & e_{22} & e_{23} \\ e_{31} & e_{32} & e_{33} \end{bmatrix} \quad (A10)$$

Também o tensor T_e pode ser expresso como um somatório de um tensor *hidrostático* mais outro *de desvio*, ou seja

$$T_e = \frac{1}{3} e_v d_{ij} + E_{ij} \quad (A11)$$

onde $e_v = e_{11} + e_{22} + e_{33}$ é a deformação volumétrica. Assim,

$$T_e = \frac{1}{3} e_v d_{ij} \begin{bmatrix} 1 & 0 & 0 \\ 0 & 1 & 0 \\ 0 & 0 & 1 \end{bmatrix} + \begin{bmatrix} E_{11} & E_{12} & E_{13} \\ E_{21} & E_{22} & E_{23} \\ E_{31} & E_{32} & E_{33} \end{bmatrix} \quad (A12)$$

Invariantes do tensor de tensões – Obtidos a partir do próprio tensor das deformações de acordo com

$$I_1 = tr(e) = e_{11} + e_{22} + e_{33} = e_v \quad (A13)$$

$$I_2 = \frac{1}{2} tr(I)^2 \quad (A14)$$

$$I_3 = \frac{1}{3} tr(I)^3 \quad (A15)$$

Planos e tensões octaédricas - Os planos octaédricos são determinados pela aplicação da condição de que os cossenos dos ângulos diretores, formados com as direções principais σ_1 , σ_2 , σ_3 , são iguais em valor absoluto. Existem oito planos que satisfazem esta condição, chamados de planos octaédricos (figura A2).

A tensão normal atuante nestes planos é calculada como

$$s_{oct} = \frac{1}{3}(s_{11} + s_{22} + s_{33}) = \frac{1}{3} J_1 \quad (A16)$$

e a tensão cisalhante octaédrica por

$$t_{oct} = \frac{1}{3} \left[(s_{11} - s_{22})^2 + (s_{11} - s_{33})^2 + (s_{22} - s_{33})^2 \right] = \sqrt{\frac{2}{3} J_{2D}} \quad (A17)$$

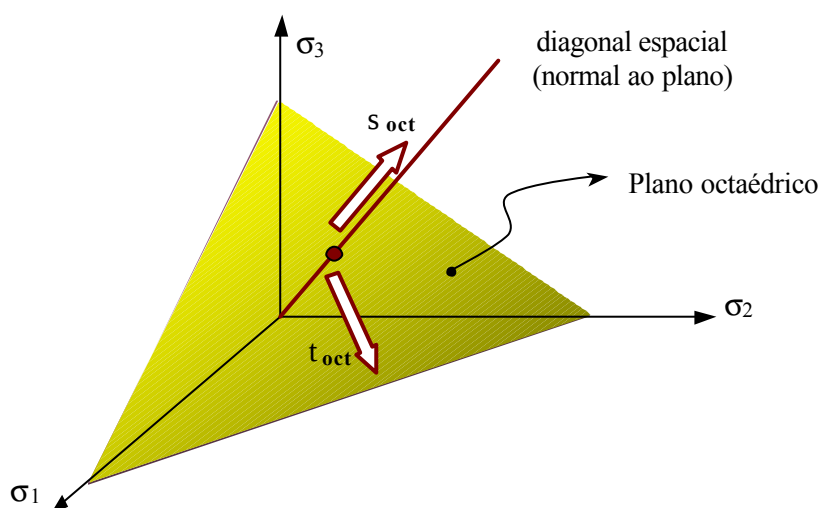


Figura A2: Um dos oito planos octaédricos e as tensões octaédricas nele atuantes.

Espaço de tensões e trajetórias de tensão - O espaço Cartesiano definido pelos eixos das tensões principais σ_1 , σ_2 , σ_3 é conhecido como *espaço das tensões principais* ou *espaço de Westergaard*, onde são representados planos relevantes à análise das trajetórias de tensão (figura A3).

Um plano relevante contido no espaço de tensões é o *plano triaxial*, ou de *Rendulic*, definido pela condição $s_2 = s_3$, própria dos ensaios triaxiais convencionais. Nesse plano, são descritas todas as trajetórias de tensão levadas a cabo

nestes ensaios. Outro plano relevante é o *octaédrico* cuja normal é paralela à diagonal principal do espaço de tensões ($s_1 = s_2 = s_3$). Particularmente, o plano octaédrico que passa pela origem, denominado de *plano p* ($s_1 + s_2 + s_3 = 0$), é bastante utilizado como plano de projeção no estudo das trajetórias de tensão espaciais.

A figura A3 apresenta também as trajetórias de tensão mais empregadas em ensaios geotécnicos, as quais são detalhadas a seguir:

- Trajetórias contidas no plano triaxial:

CH: Compressão hidrostática ($\Delta s_1 = \Delta s_2 = \Delta s_3$).

CTC: Compressão triaxial convencional ($\Delta s_1 > 0$, $\Delta s_2 = \Delta s_3 = 0$).

RTC: Compressão triaxial reduzida ($\Delta s_1 = 0$, $\Delta s_2 = \Delta s_3 < 0$).

CTE: Extensão triaxial convencional ($\Delta s_1 = 0$, $\Delta s_2 = \Delta s_3 > 0$).

RTE: Extensão triaxial reduzida ($\Delta s_1 < 0$, $\Delta s_2 = \Delta s_3 = 0$).

- Trajetórias contidas no plano octaédrico:

CS: cisalhamento simples ($\Delta s_2 = 0$, $\Delta s_1 = -\Delta s_3$).

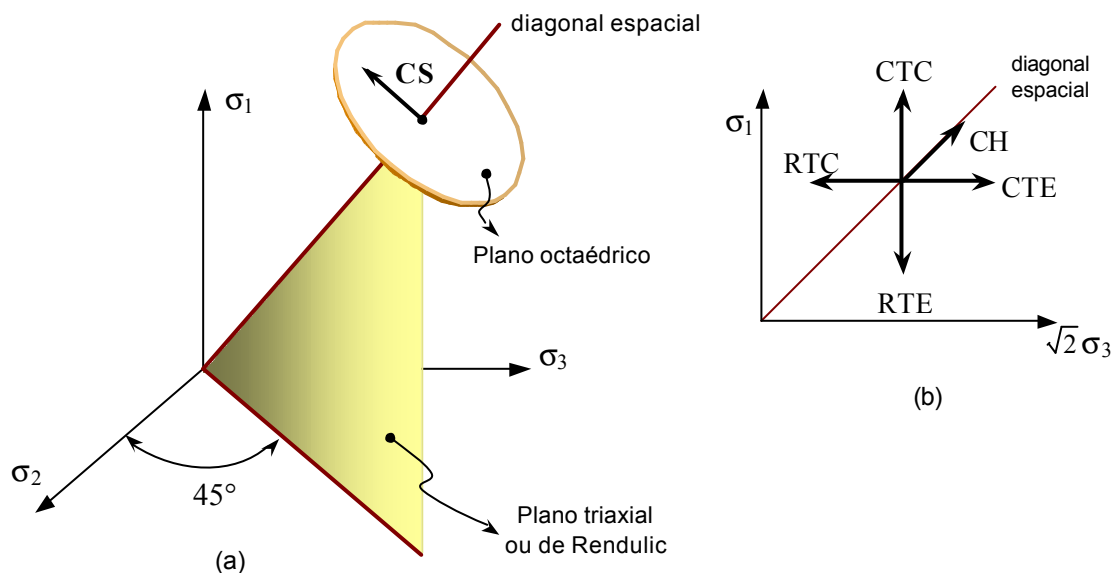


Figura A3: Espaço de tensões: a) Espaço de Westergaard, plano triaxial e octaédrico; b) Trajetórias de tensão no plano triaxial.